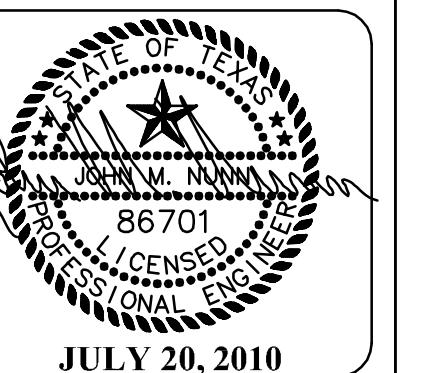
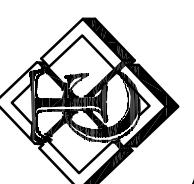


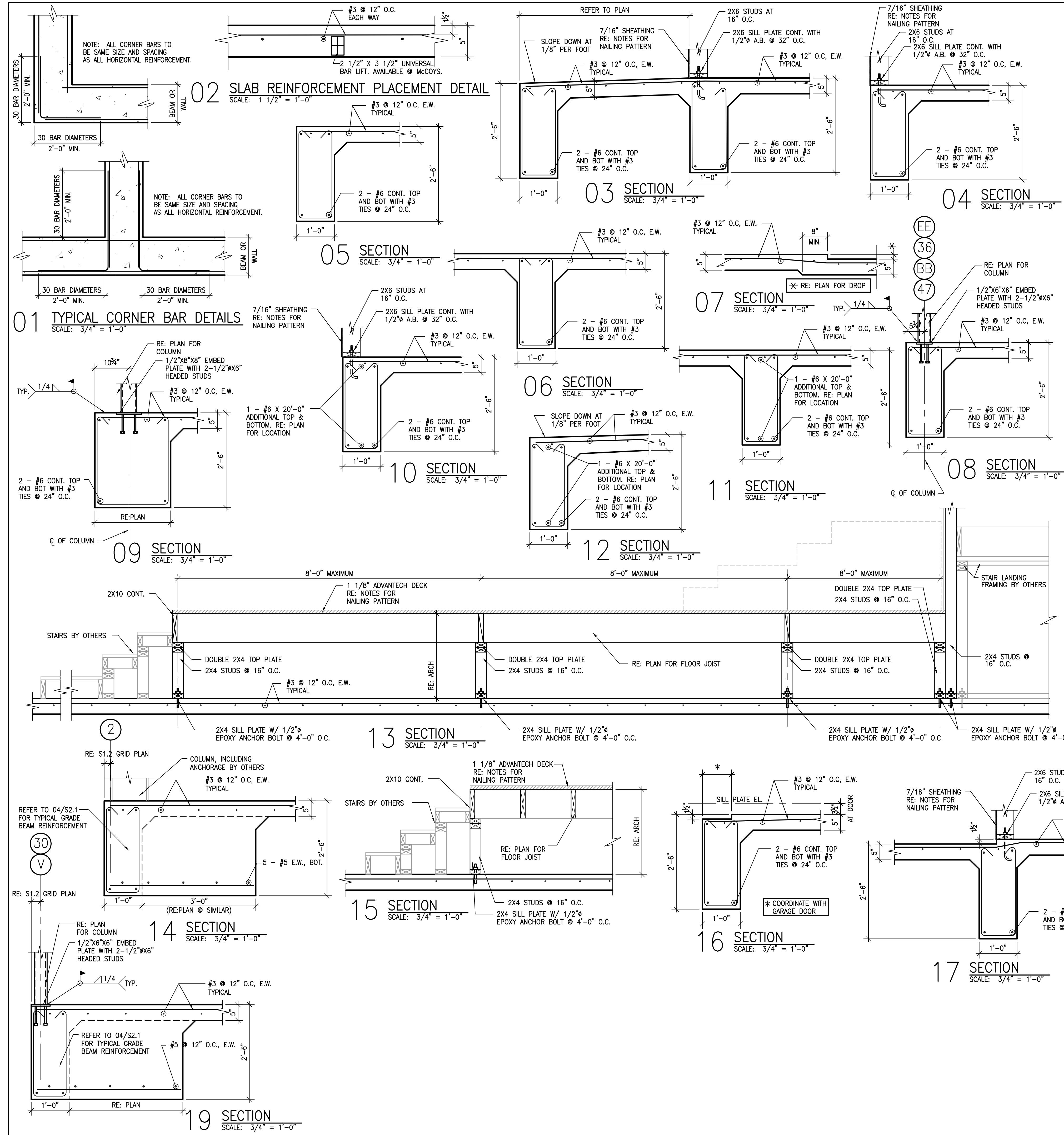
TRUE VISION CHURCH
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SHEET NUMBER

S1.2



CODE & DESIGN SPECIFICATIONS:

GENERAL: 2006 INTERNATIONAL BUILDING CODE IS USED AS THE BASIC CODE DOCUMENT. THIS IS SUPPLEMENTED BY THE FOLLOWING ADDITIONAL CODES AND REFERENCES TO BE USED FOR DESIGN, DETAILING AND CONSTRUCTION.

- STRUCTURAL STEEL: 1989 SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS OF AISC (NINTH EDITION).
- STRUCTURAL CONCRETE: 2002 BUILDING CODE FOR REINFORCED CONCRETE OF THE AMERICAN CONCRETE INSTITUTE (ACI 318-2002).
- STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, K-SERIES (SJI) AND ASC 1997.

DESIGN LOADS: (LIVE LOAD)

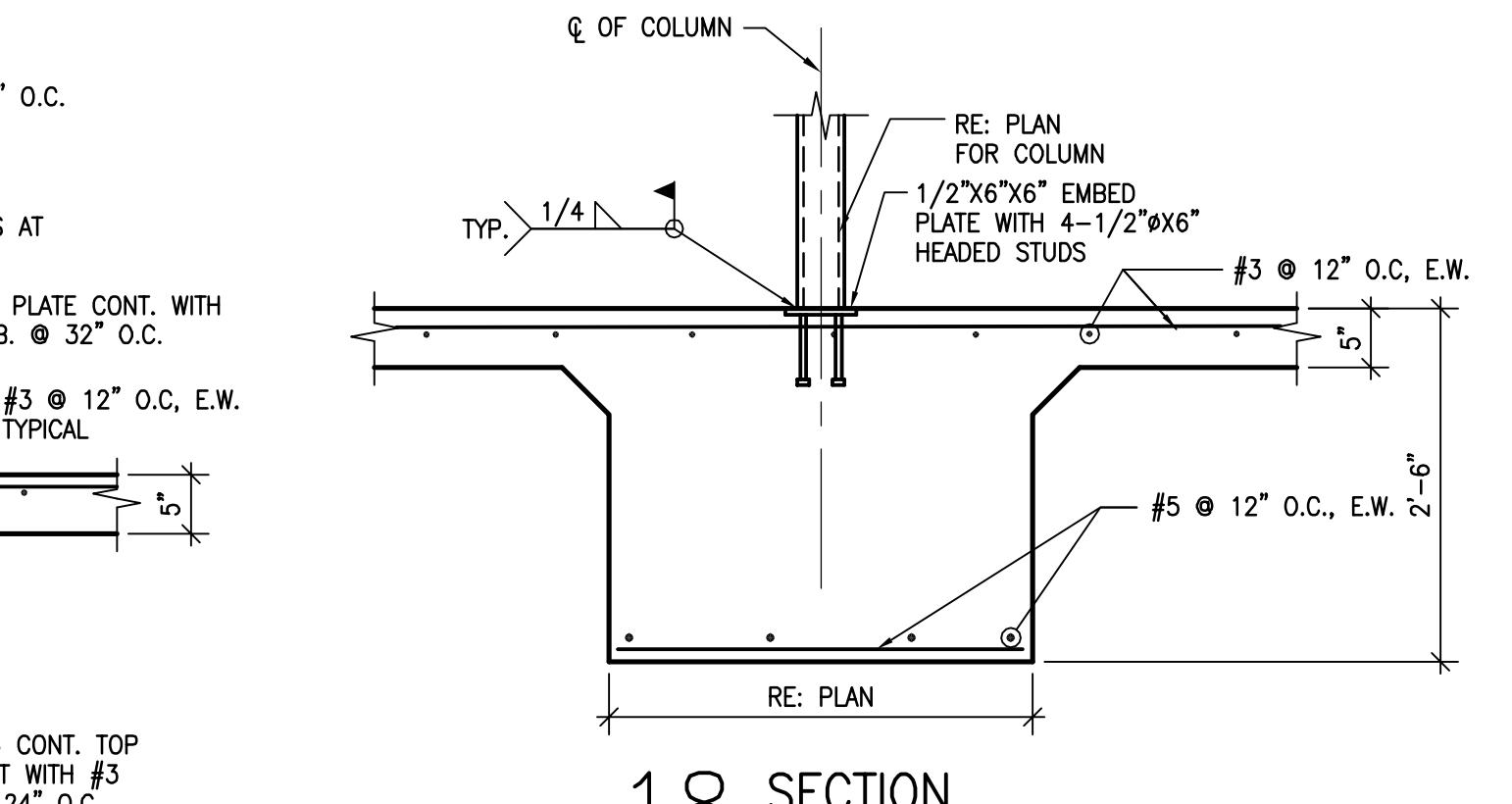
SOIL BEARING 2000 PSF

GENERAL CONDITIONS:

- THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND DETAILS BEFORE FABRICATION OR CONSTRUCTION AND REPORT ANY DISCREPANCIES OR OMISSIONS IN THE CONTRACT DOCUMENTS TO THE STRUCTURAL ENGINEER.
- THE CONTRACTOR SHALL COORDINATE ALL LEAVE-OUTS, SLEEVES AND OTHER SLAB PENETRATIONS BEFORE CONSTRUCTION.
- THE CONTRACTOR SHALL NOT PROCEED WITH FABRICATION OF STRUCTURAL ELEMENTS WITHOUT PRIOR REVIEW OF ALL SHOP DRAWINGS BY THE STRUCTURAL ENGINEER.
- THE STRUCTURAL ENGINEER'S REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS SHALL NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY FOR DEVIATIONS FROM THE CONTRACT DOCUMENTS, NOR FROM ANY ERRORS IN THE SHOP DRAWINGS.

STRUCTURAL STEEL:

- STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", 1989 EDITION.
- STRUCTURAL STEEL TO CONFORM TO ASTM A992-50 GRADE STEEL, UNLESS OTHERWISE NOTED.
- STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B.
- WELDING SHALL BE PER THE LATEST AWS STANDARDS WITH E70XX ELECTRODES. ALL WELDS IN FIELD TO LABORATORY INSPECTED. ALL FIELD WELDS TO BE DONE BY CERTIFIED WELDERS.
- PROVIDE FILLET WELDS AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.
- ALL WELDS TO EXISTING MEMBERS SHALL BE $3/16"$ FILLET WELDS ALL AROUND UNLESS NOTED OTHERWISE.
- BOLTS AND BOLTED CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATIONS FOR STRUCTURAL JOINTS USING A.S.T.M. A325 BOLTS OR A490 BOLTS" AS APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED JOINTS USING A325 BOLTS. USE FRICTION TYPE BOLTS WITH WASHERS.
- SHREAP CAPACITY OF CONNECTIONS (BOTH WELDED AND BOLTED) SHALL NOT BE LESS THAN THE NOTED SHEAR OR 70% OF THE ALLOWABLE BEAM LOAD AS TABULATED IN THE A.I.S.C. MANUAL OF STEEL CONSTRUCTION, PART 2, FOR SIZE AND SPAN, WHICHEVER IS LARGER.
- UNLESS OTHERWISE NOTED, ALL STEEL SHALL BE SHOP PRIMED WITH FAST-CURING LEAD AND CHROMATE-FREE UNIVERSAL MODIFIED ALKYD PRIMER COMPLYING WITH PERFORMANCE REQUIREMENTS OF FS TT-P-664 SELECTED FOR GOOD RESISTANCE TO NORMAL ATMOSPHERIC CORROSION. ALL STEEL AND BRICK LINTEL ANGLES EXPOSED TO THE ELEMENTS SHALL BE GALVANIZED TO CONFORM WITH G-60.
- DO NOT PAINT STEEL THAT IS EMBEDDED IN CONCRETE.



HILL COUNTRY STRUCTURAL, INC.
STRUCTURAL ENGINEERING CONSULTANTS

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jm@hillcountrystructural.com
TEXAS REGISTERED ENGINEERING FIRM #7388

SHEET NUMBER
S2.1

TIMBERCON CONSTRUCTION, INC.
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REVISIONS:		
DATE	BY	

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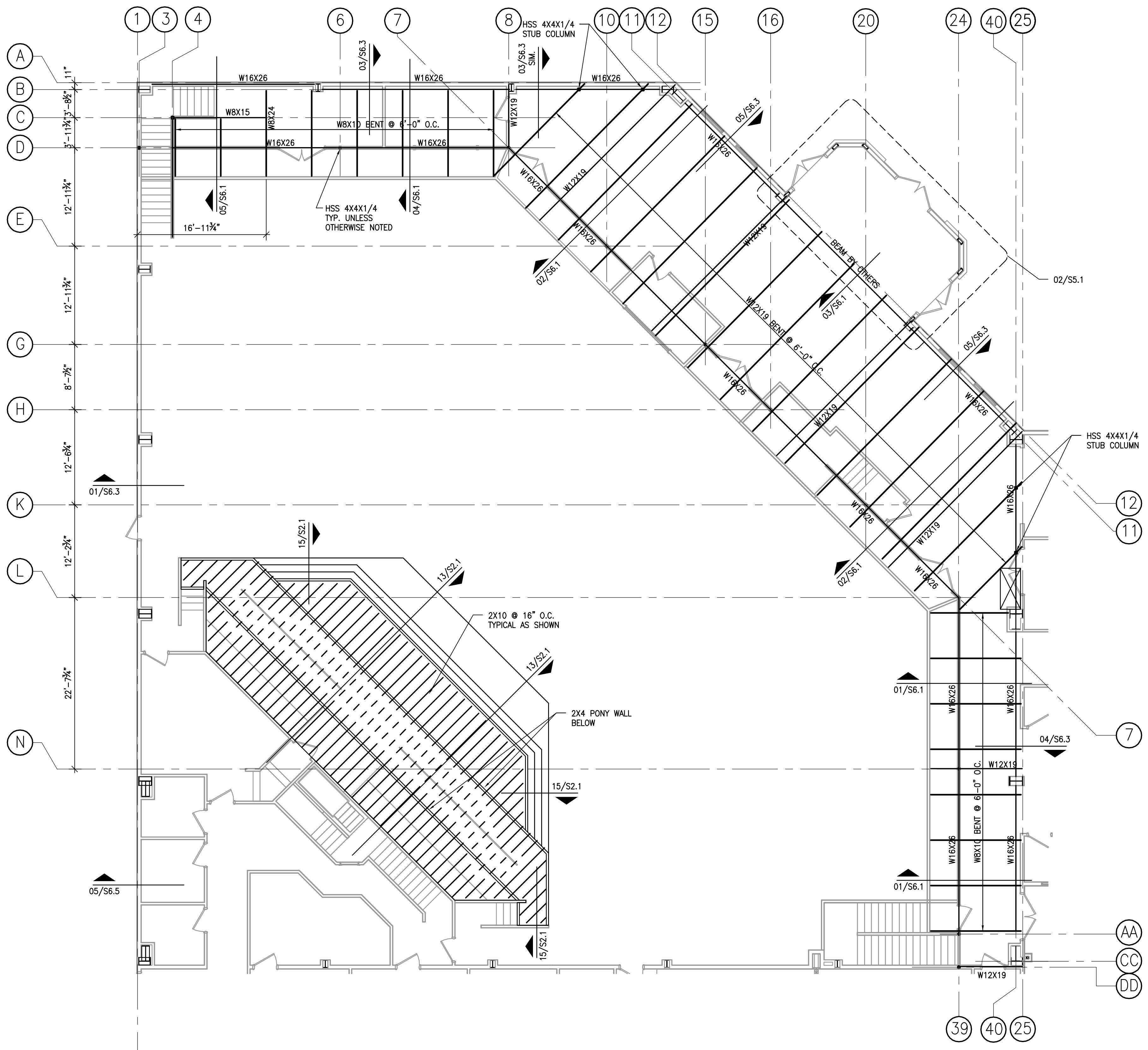
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BALCONY & STAGE FRAMING PLAN

SCALE: 1/8" = 1'-0"



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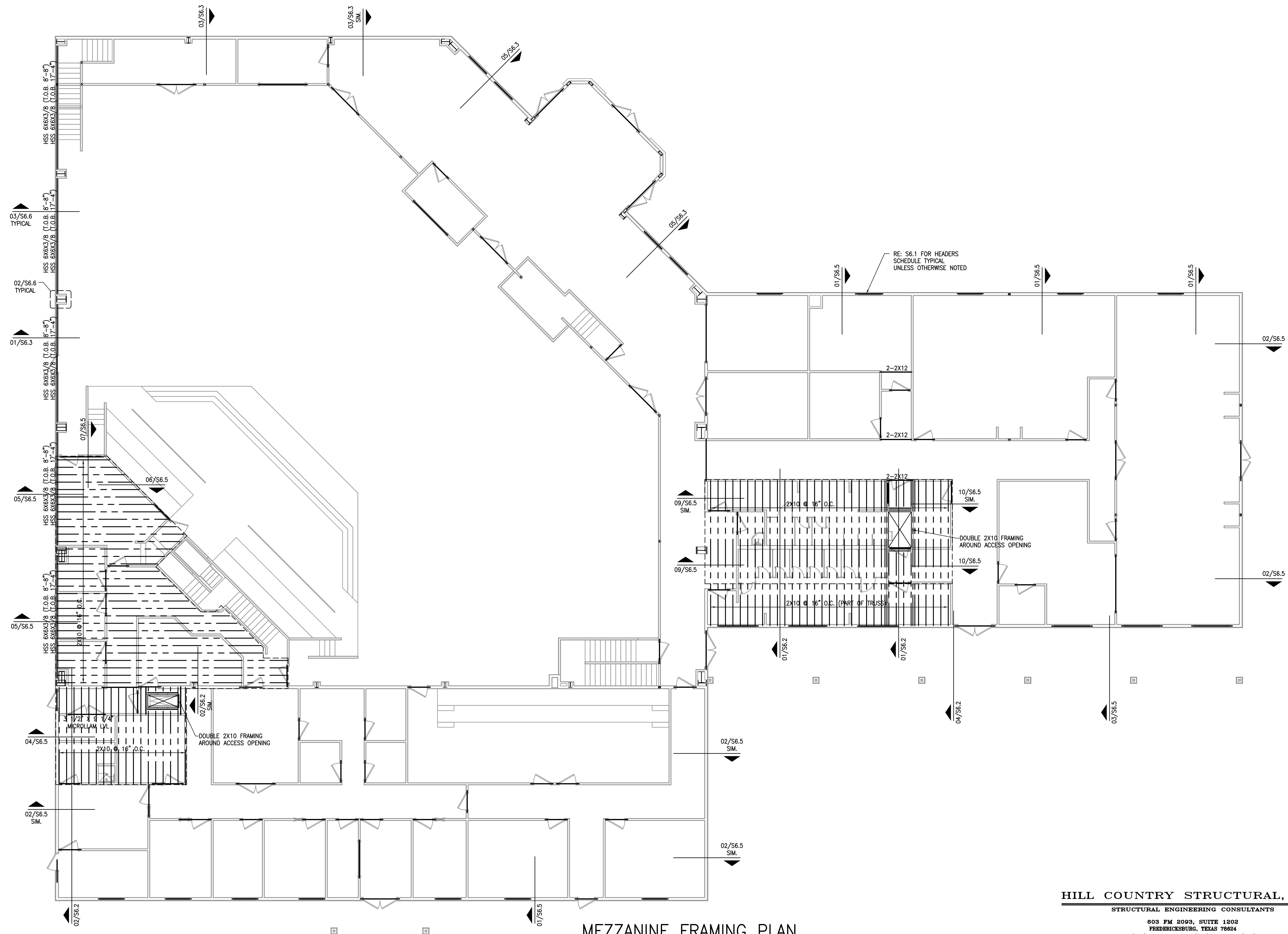
603 FM 2083 SUITE 1202

603 FM 2093, SUITE 1202
FREDERICKSBURG, TEXAS 78624

jmehillcountrystructural.com
TEXAS REGISTERED ENGINEERING FIRM F-7838

SHEET NUMBER

S3.1



HILL COUNTRY STRUCTURAL, INC.
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TEXAS REGISTERED ENGINEERING FIRM F-7838

SHEET NUMBER

S3.2



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MEZZANINE FRAMING PLAN

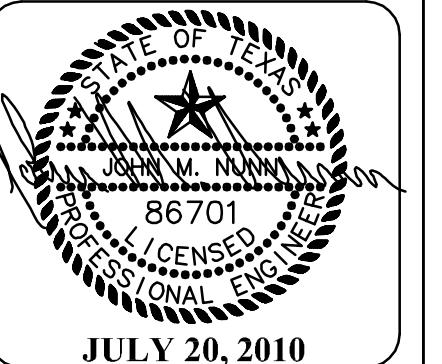
REVISIONS:		
DATE	BY	

REVISIONS:		
DATE	BY	

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ROOF FRAMING PLAN

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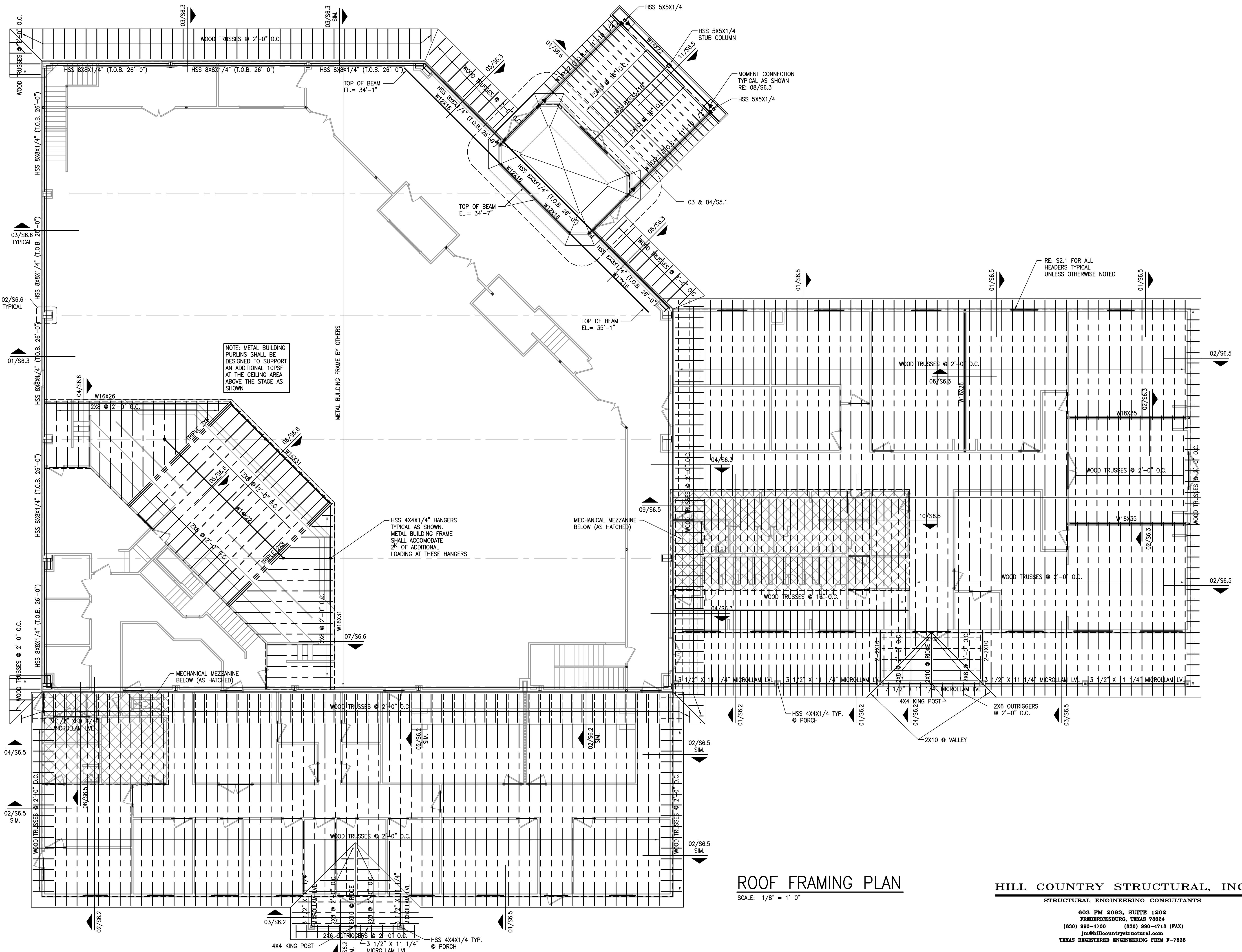


SHEET NUMBER

S4.1

ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

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jmc@hillcountrystructural.com
TEXAS REGISTERED ENGINEERING FIRM F-7838

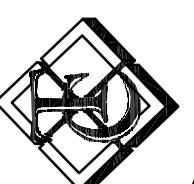


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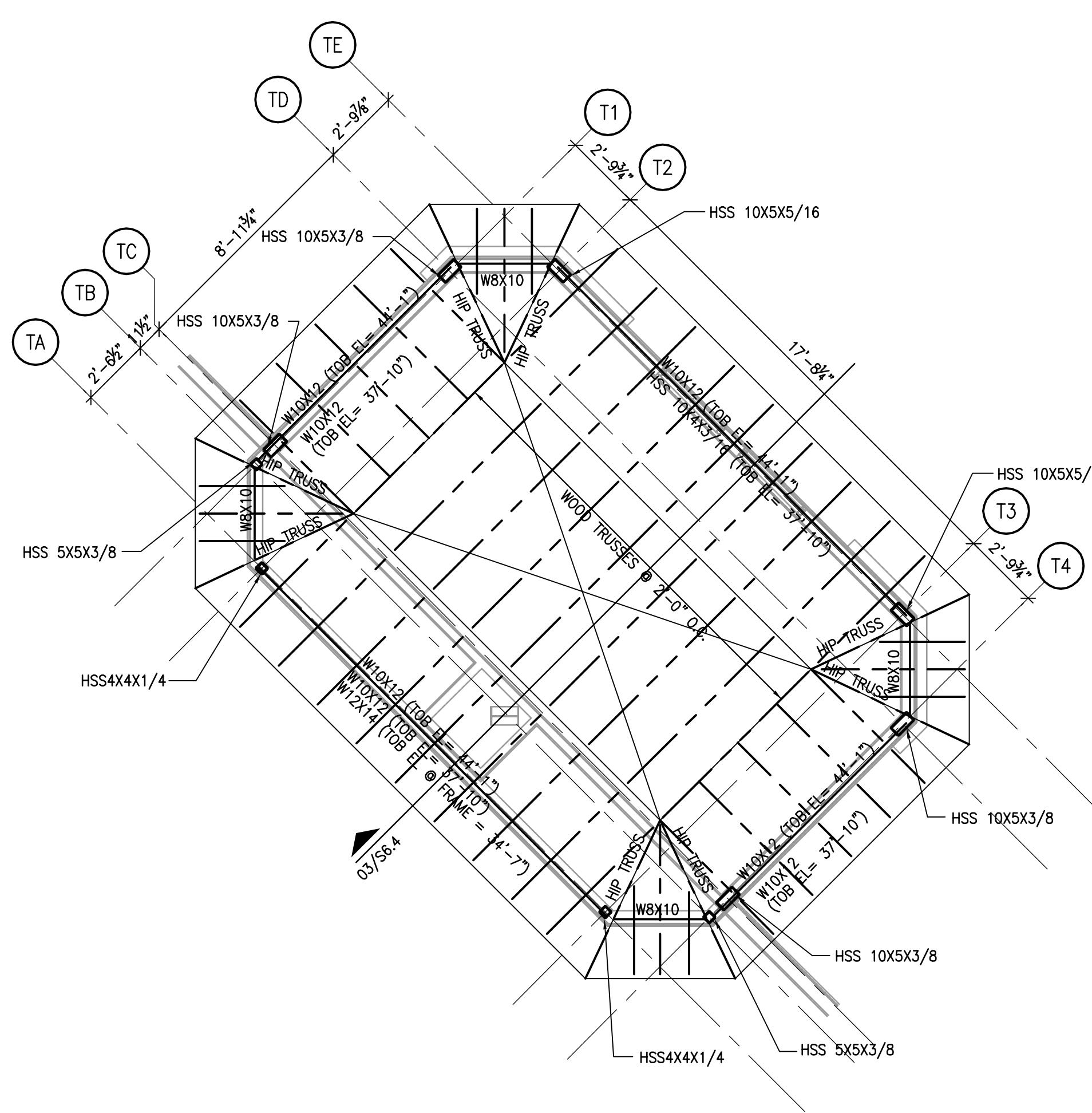
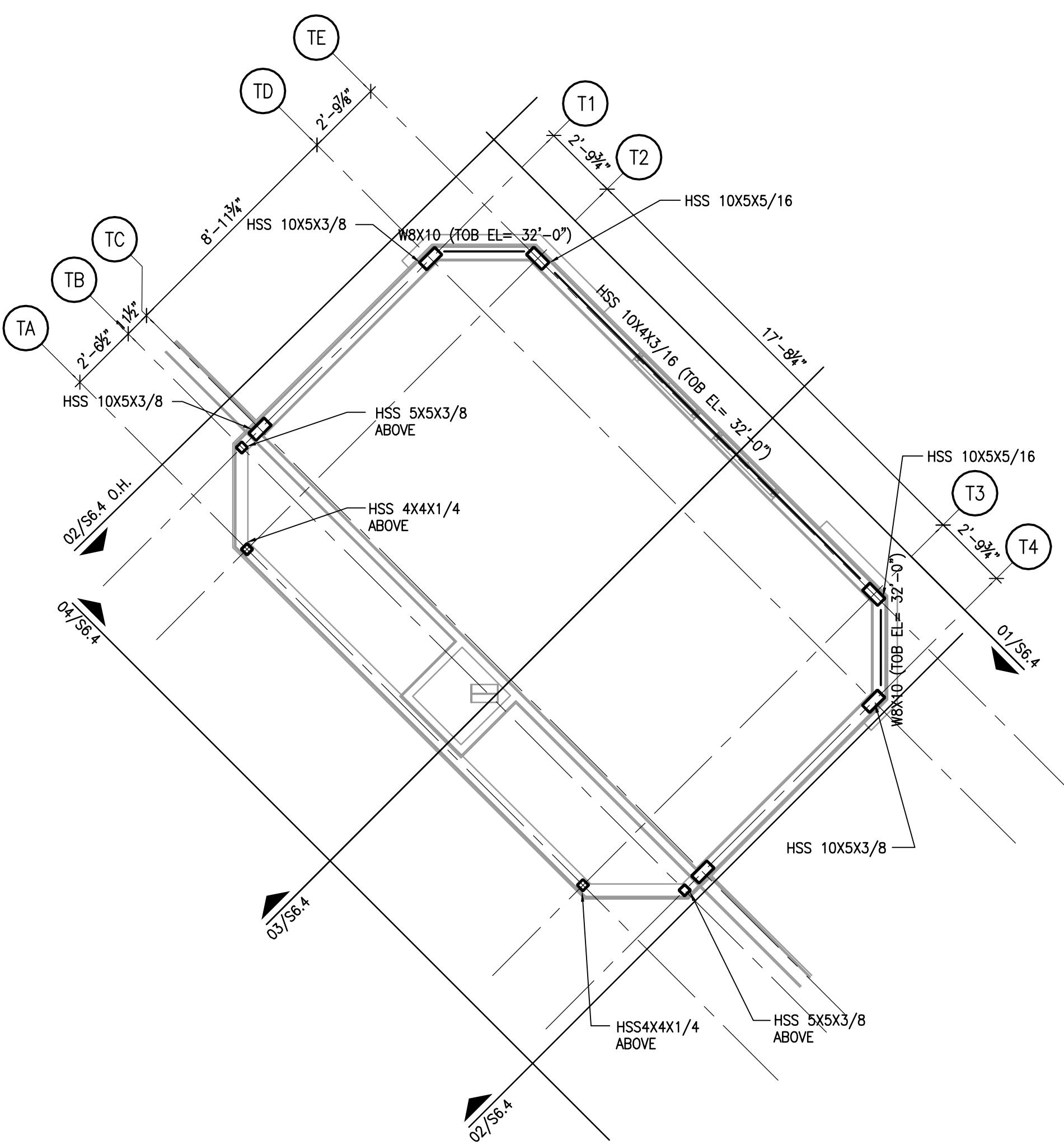
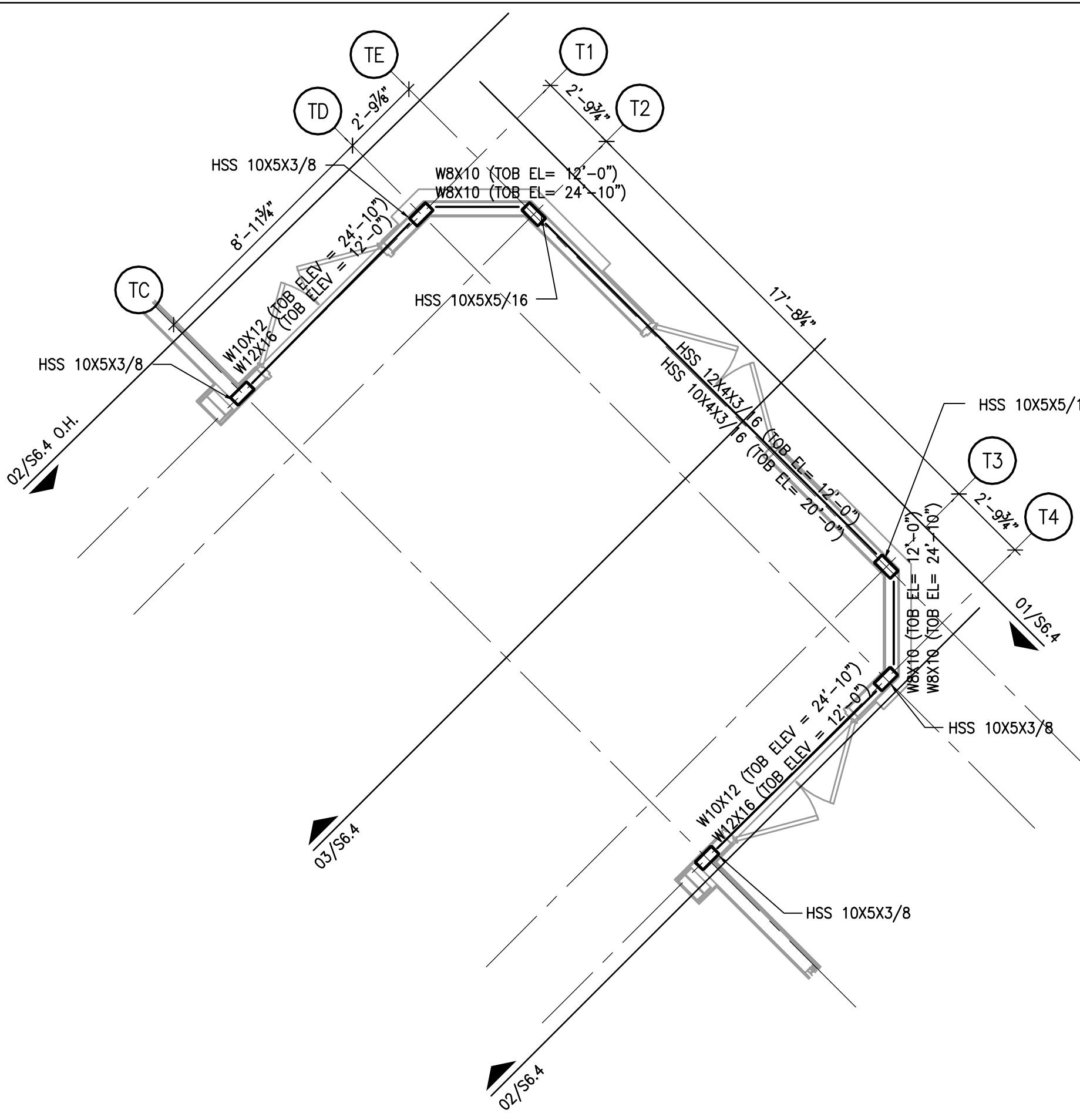
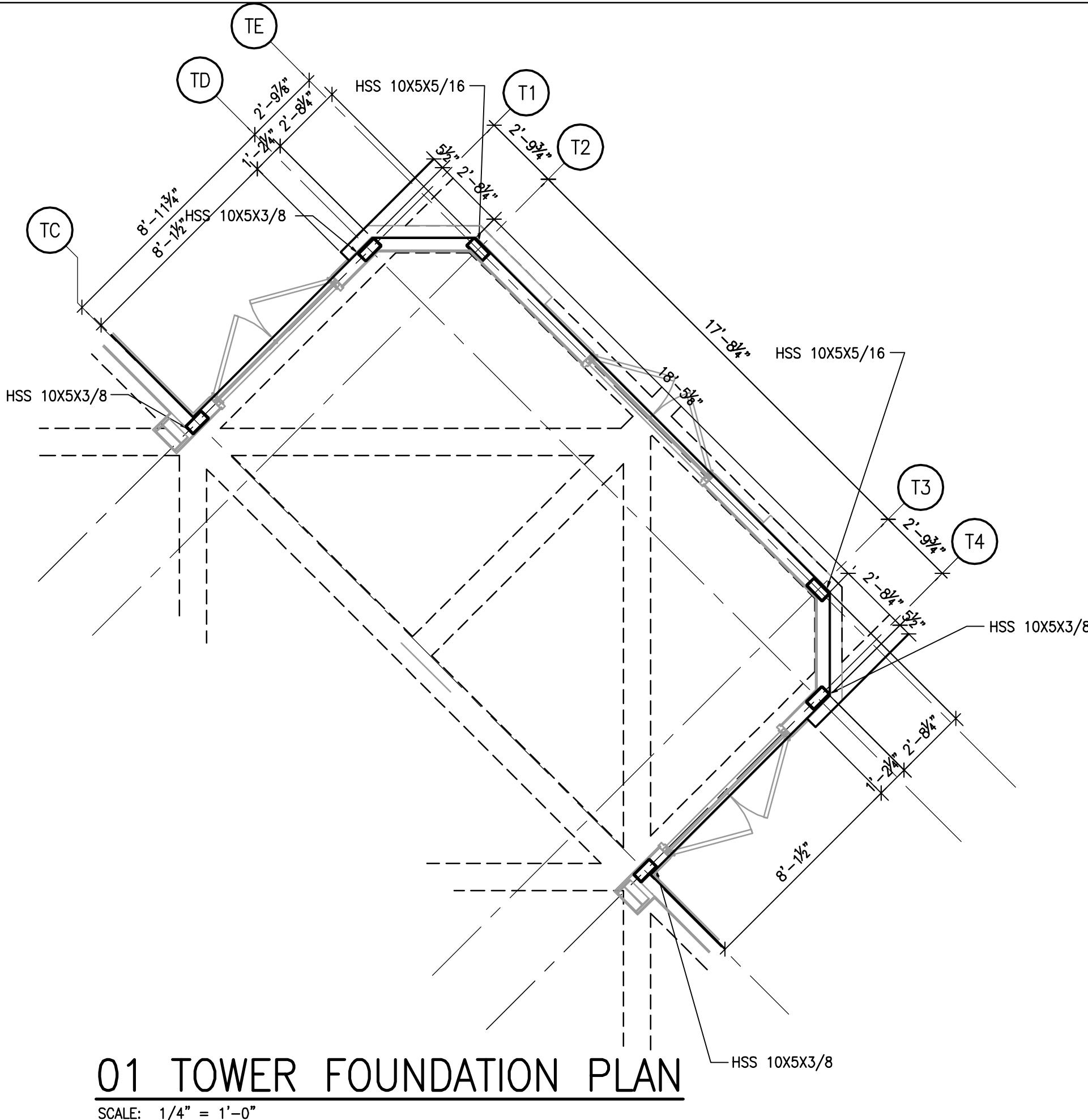
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ACKERMAN ROAD
SAN ANTONIO, TX 78219

TOWER FRAMING PLAN

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SHEET NUMBER
S5.1



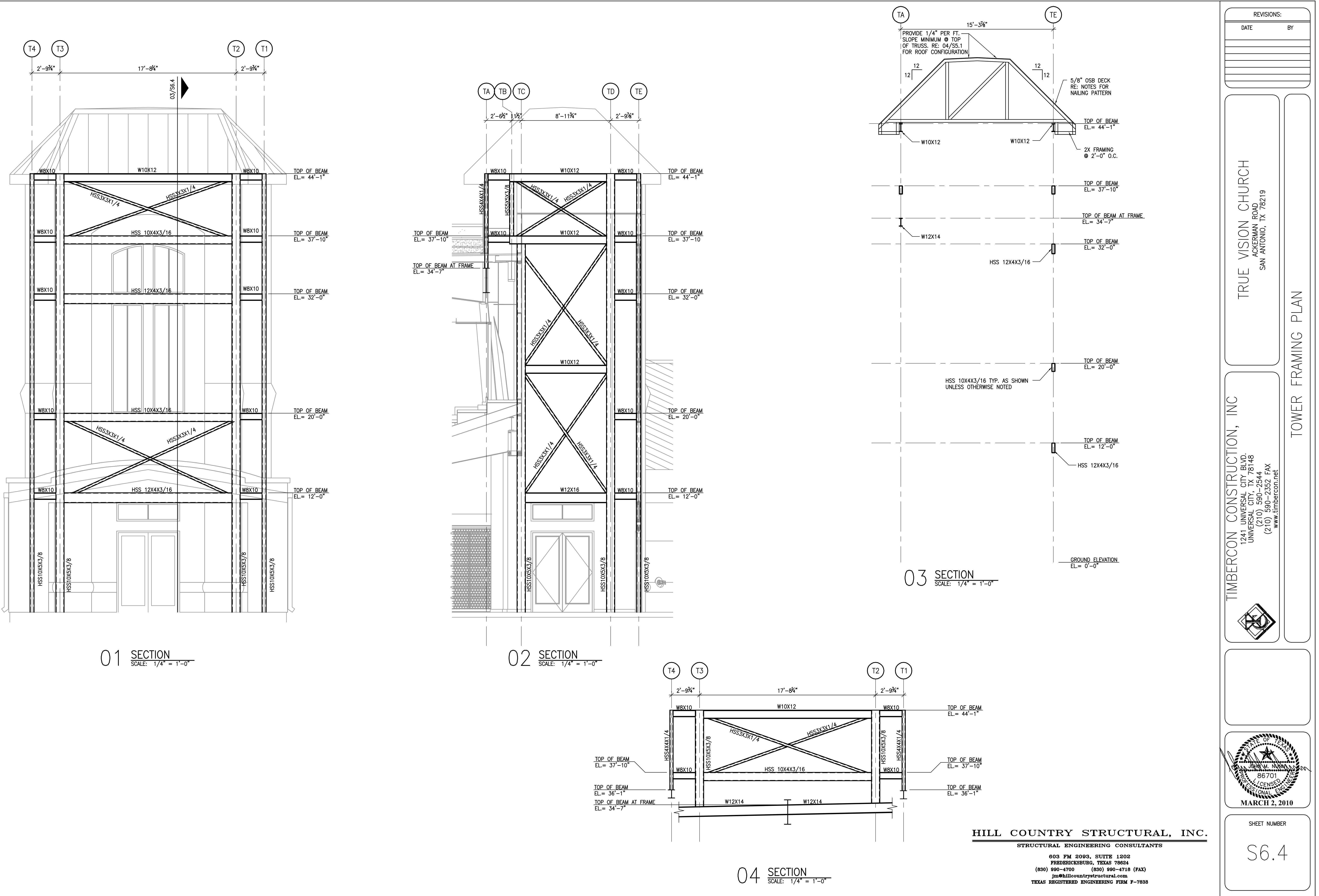
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jm@hillcountrystructural.com
TEXAS REGISTERED ENGINEERING FIRM F-7838

REVISIONS:

DATE BY

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DATE BY



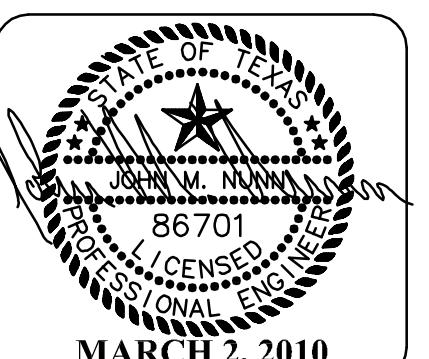
REVISIONS:

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FRAMING SECTIONS

TIMBERCON CONSTRUCTION, INC.
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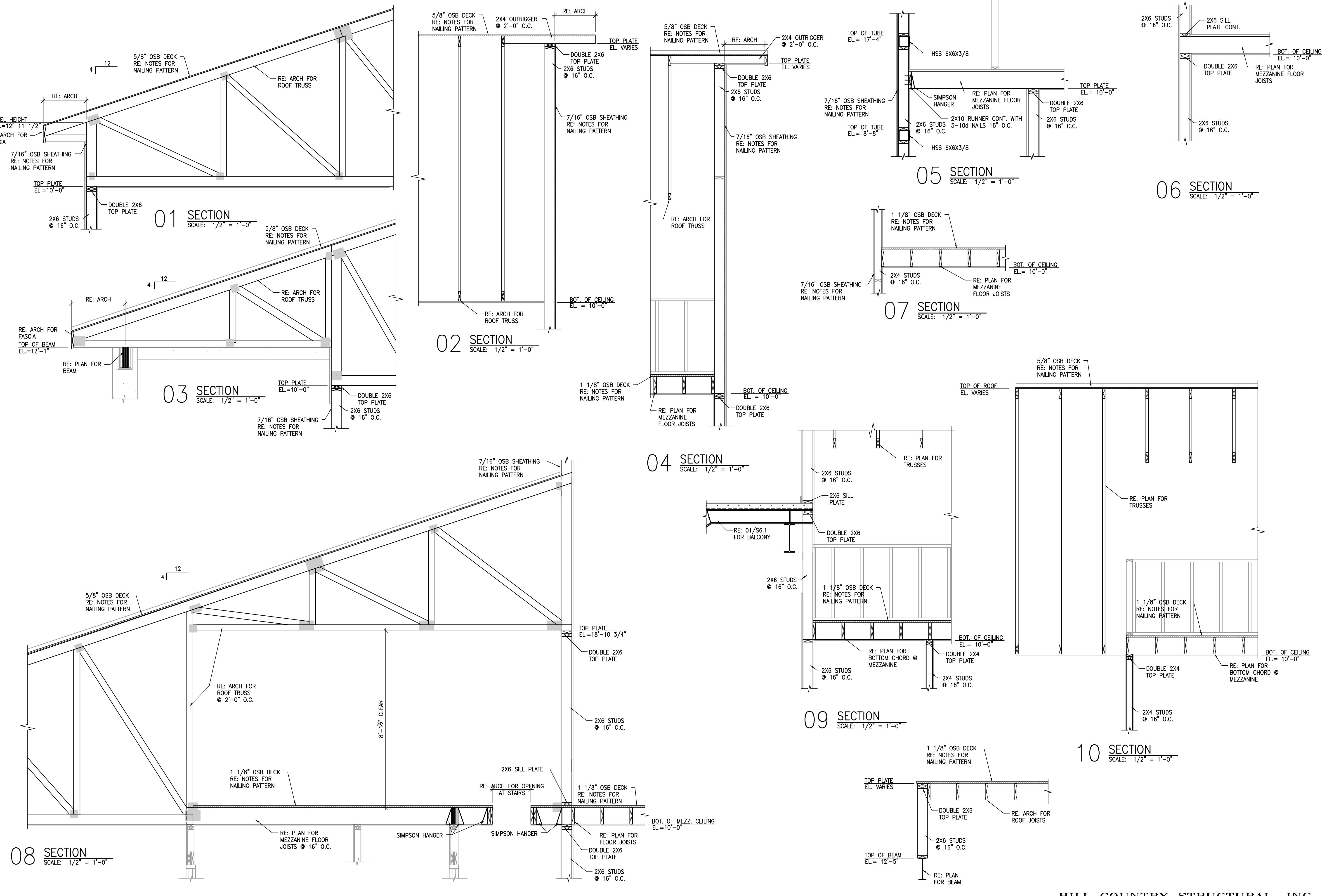
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S6.5

HILL COUNTRY STRUCTURAL, INC.

STRUCTURAL ENGINEERING CONSULTANTS

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jm@hillcountrystructural.com
TEXAS REGISTERED ENGINEERING FIRM F-7838



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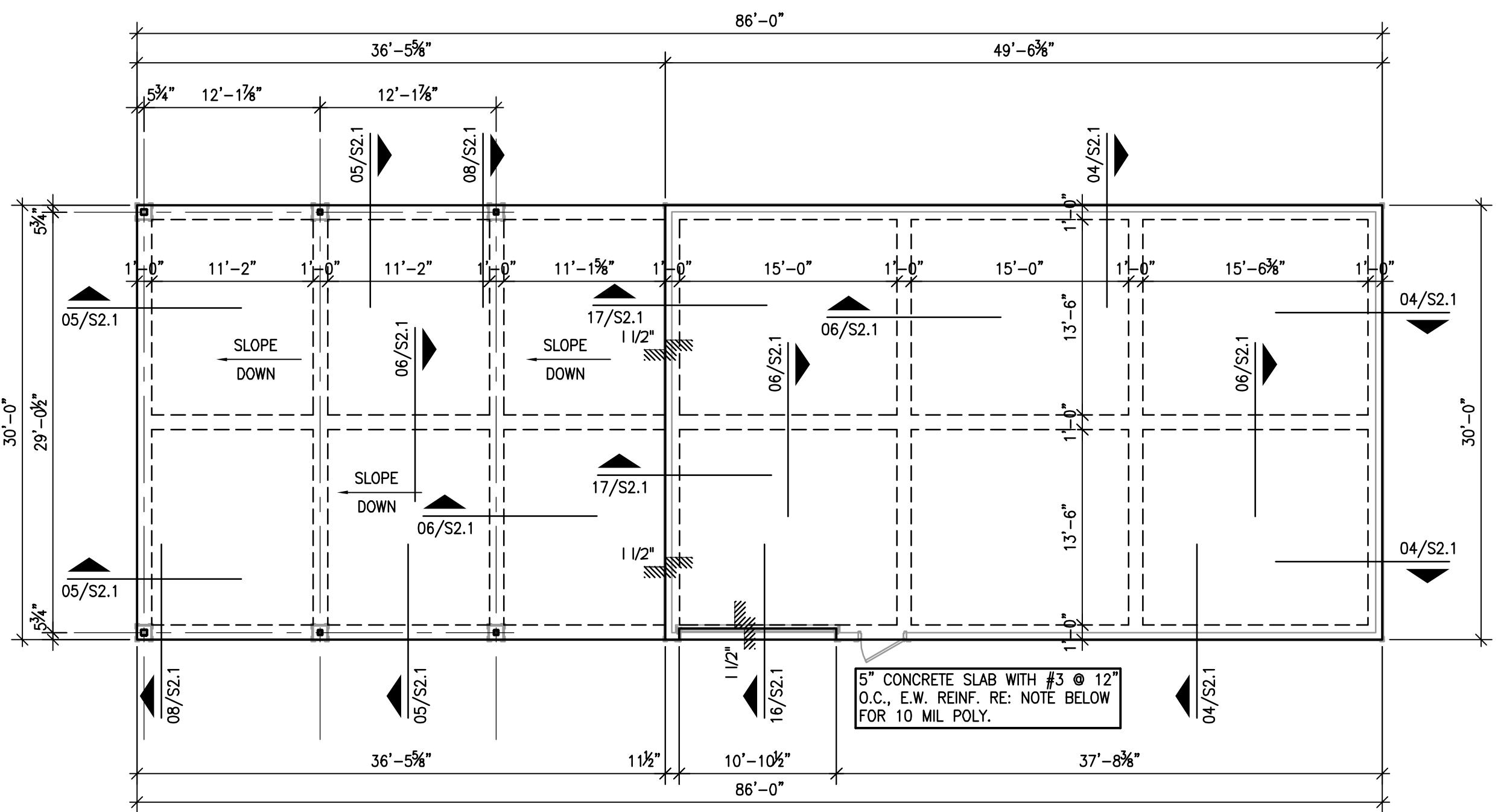
DATE BY

REVISIONS:
DATE BY

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SAN ANTONIO, TX 78219

GARAGE FOUNDATION & FRAMING PLANS

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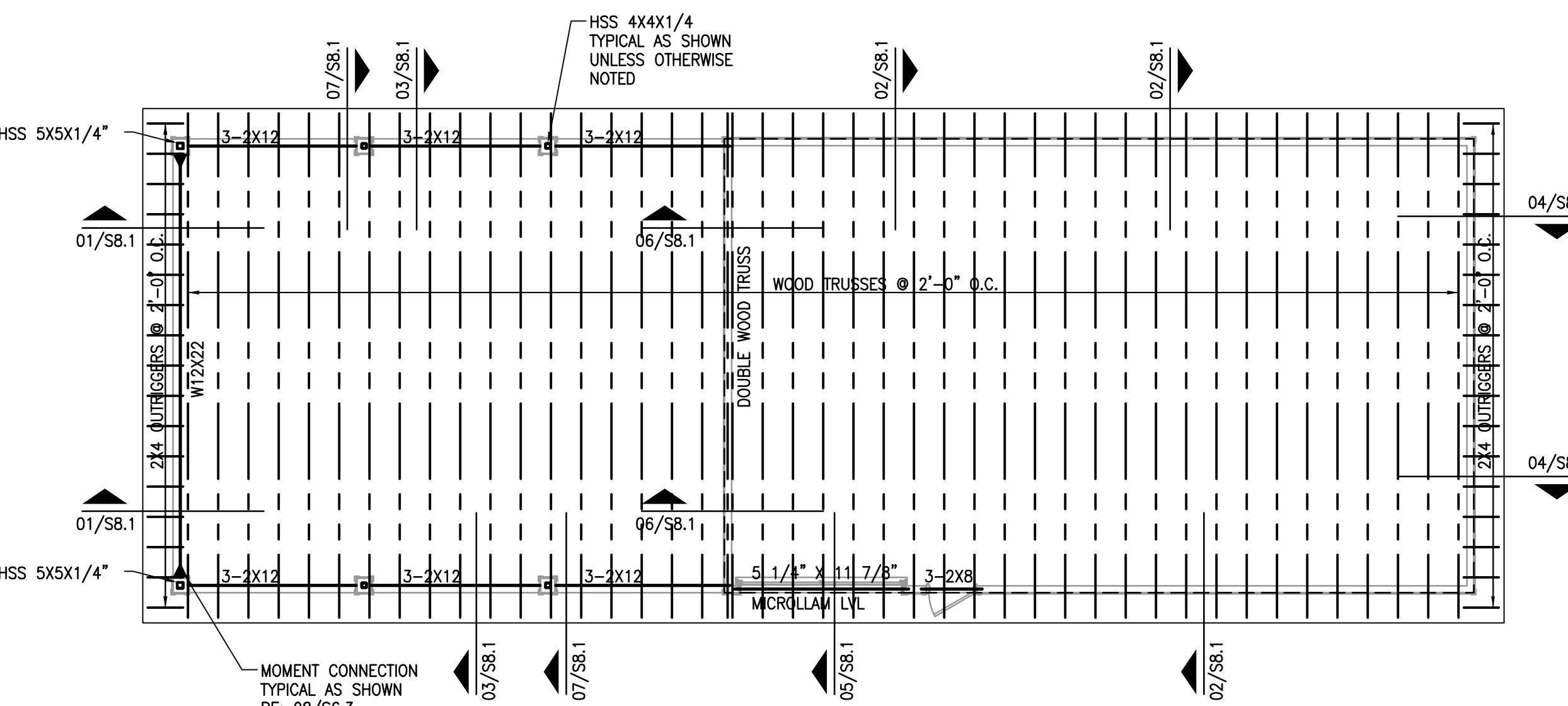
FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTE: VAPOR RETARDER SHALL BE 10 MIL MIN. WITH PERMEANCE OF LESS THAN 0.3 US PERMS (ASTM E 96) AND INSTALLED PER ASTM E 1634

NOTE: CONTRACTOR TO VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION INCLUDING ALL SLAB DROPS

NOTE: IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE THE PAD PER THE STRUCTURAL NOTES, AND TO SET THE FINISH FLOOR ELEVATION FOR PROPER DRAINAGE.



ROOF FRAMING PLAN

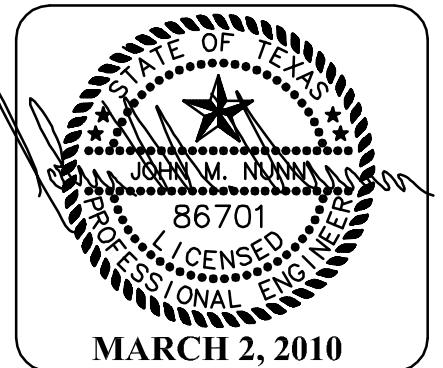
SCALE: 1/8" = 1'-0"

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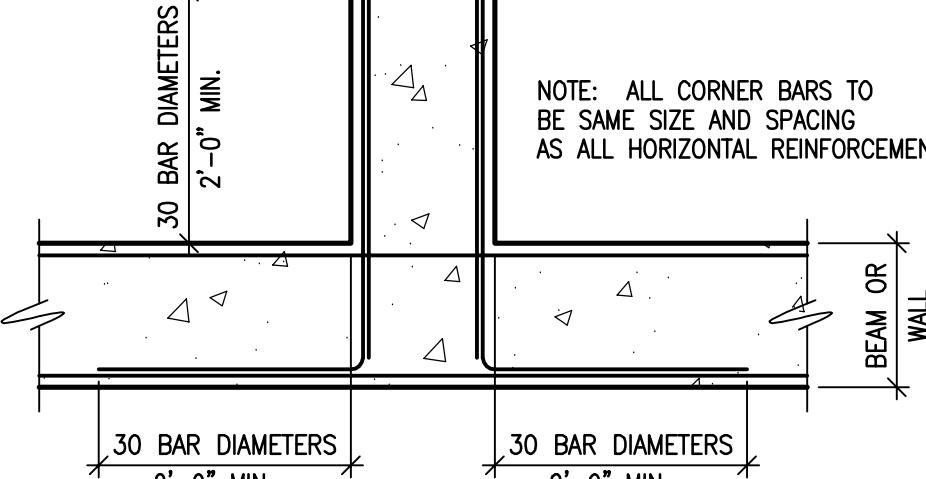
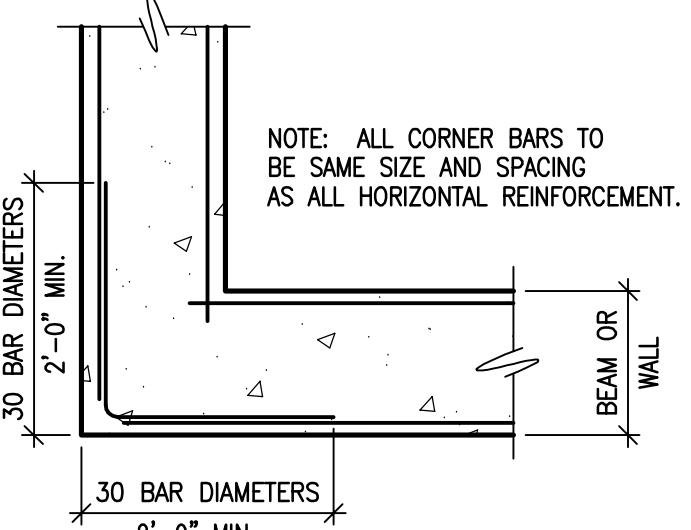
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S7.1

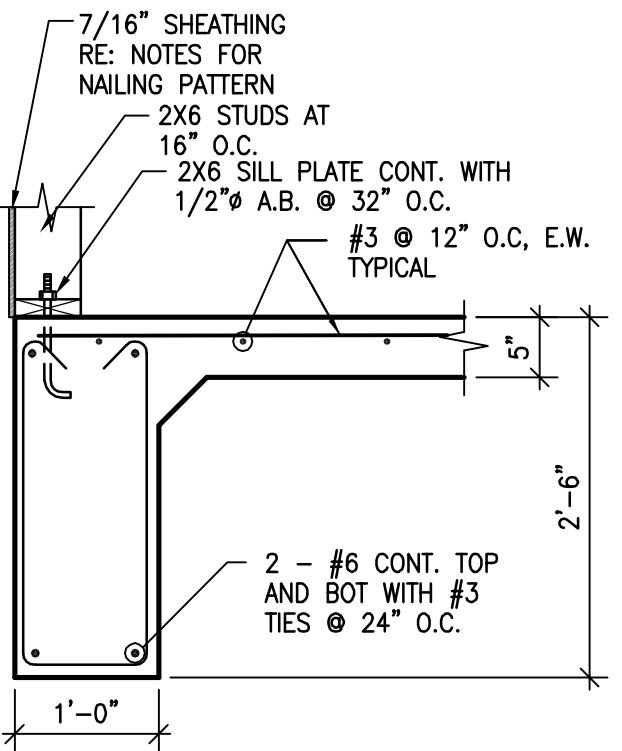


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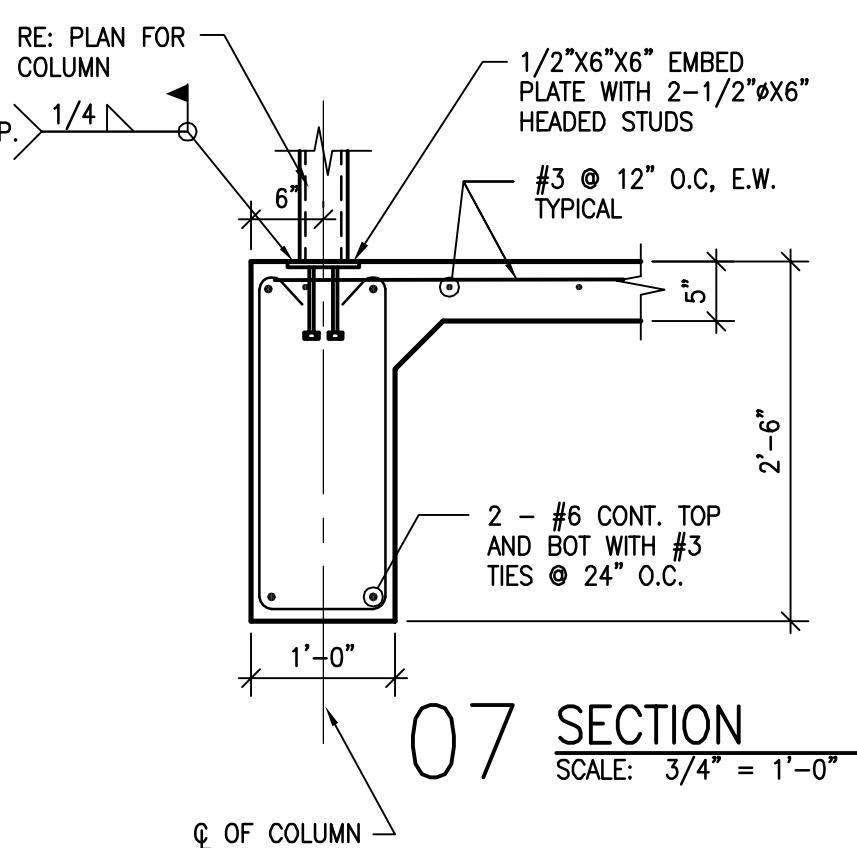
DATE BY



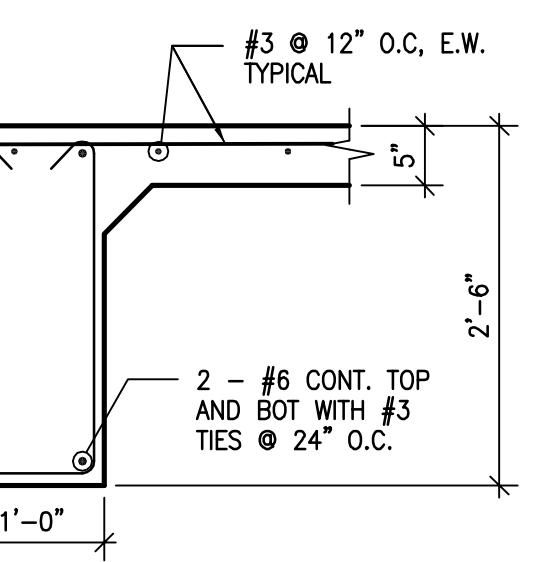
01 TYPICAL CORNER BAR DETAILS
SCALE: 3/4" = 1'-0"



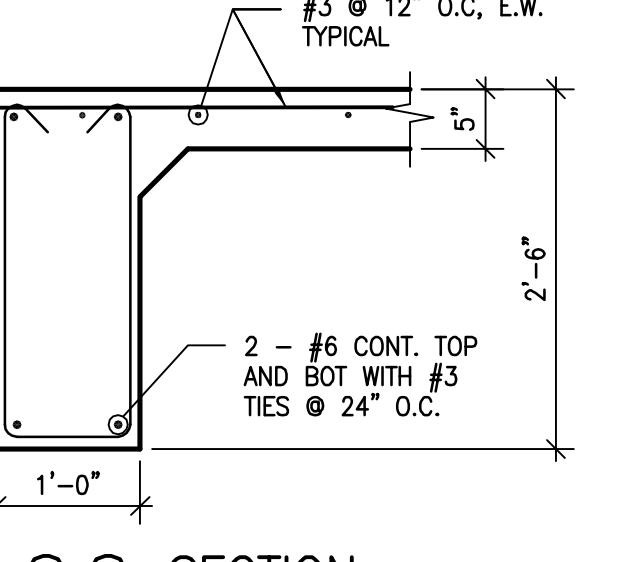
04 SECTION
SCALE: 3/4" = 1'-0"



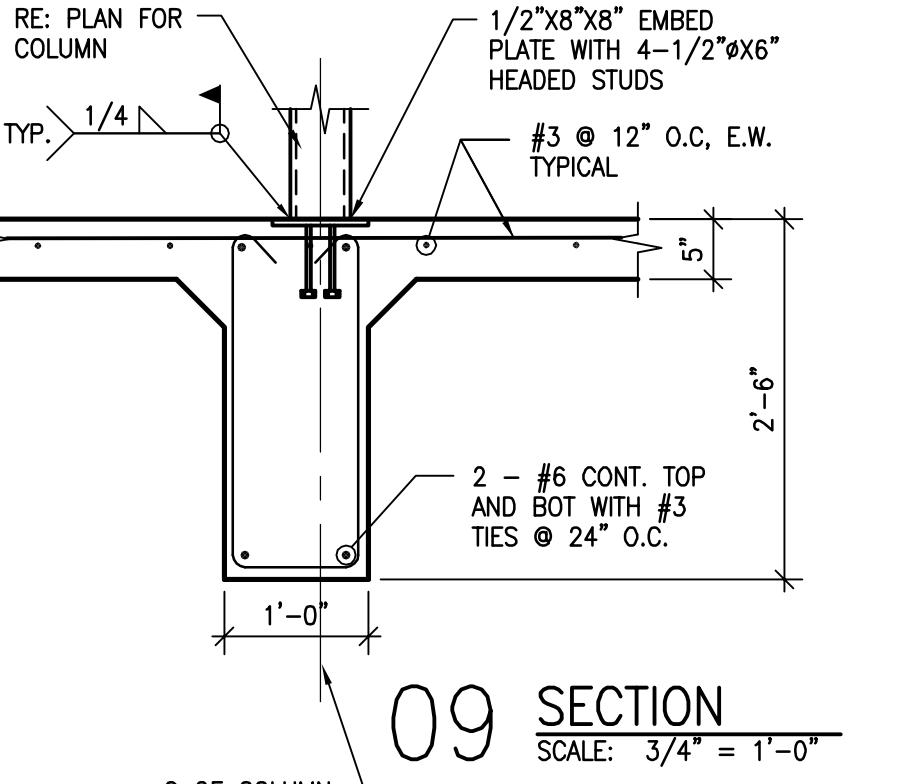
07 SECTION
SCALE: 3/4" = 1'-0"



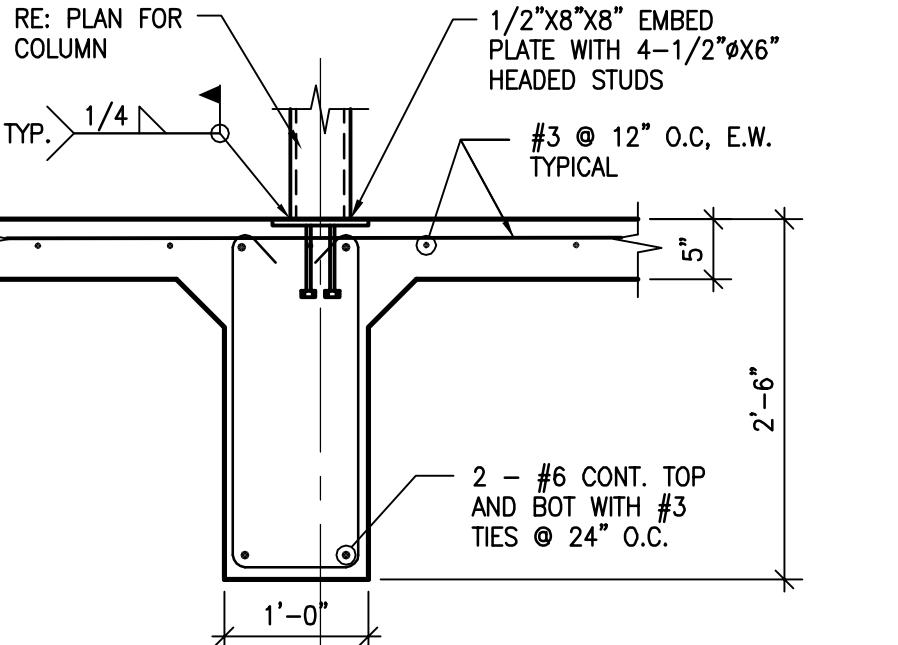
05 SECTION
SCALE: 3/4" = 1'-0"



06 SECTION
SCALE: 3/4" = 1'-0"



08 SECTION
SCALE: 3/4" = 1'-0"



09 SECTION
SCALE: 3/4" = 1'-0"

PREFABRICATED WOOD TRUSSES:

- PREFABRICATED WOOD TRUSS OUTLINES ARE SHOWN. REFER TO ARCHITECTURAL DRAWINGS FOR ROOF SLOPES AND CONTROLLING ELEVATIONS.
- TRUSSES SHALL BE DESIGNED TO CARRY THE FOLLOWING LOADS:

ROOF LIVE LOAD	20 PSF
TOP CHORD DEAD LOAD	8 PSF
BOTTOM CHORD DEAD LOAD	10 PSF
UPLIFT	12 PSF
SELF WEIGHT AND ANY APPLICABLE CONCENTRATED LOADS	
- ALL TRUSSES SHALL HAVE TRIANGULATED PANELS.
- ALL CONCENTRATED LOADS, INCLUDING REACTIONS, SHOULD OCCUR AT PANEL POINTS.
- THE MINIMUM TOP CHORD SIZE SHALL BE 2X6.
- ALL TRUSSES SHOULD HAVE HORIZONTAL RESTRAINT AT ONE END ONLY.
- THE TRUSS MANUFACTURER SHOULD SUBMIT BRACING LAYOUT FOR APPROVAL ALONG WITH TRUSS DESIGN.
- FRAMING MATERIAL SHALL BE NO. 2 GRADE KD SOUTHERN YELLOW PINE OR DOUGLAS FIR, MMG 19%. PROVIDE ALLOWABLE BENDING STRESS OF 1500 PSI WITH A MODULUS OF ELASTICITY OF 1,700,000 PSI, AS DETERMINED BY AN APPROVED LUMBER GRADING AGENCY.
- THE DESIGN AND FABRICATION OF ALL WOOD TRUSSES SHALL MEET "NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND TIMBERS", "FACINGS", BY NATIONAL FOREST PRODUCTS ASSOCIATION'S LATEST REVISIONS, "TIMBER CONSTRUCTION STANDARDS", BY AMERICAN INSTITUTE OF TIMBER CONSTRUCTION, LATEST REVISIONS, "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" BY TRUSS PLATE INSTITUTE, LATEST REVISIONS.

- THE WOOD TRUSSES SHALL BE CUSTOM DESIGNED TO FIT THE DIMENSIONS AND LOADS INDICATED ON THE PLANS. ALL DESIGNS SHALL BE IN ACCORDANCE WITH ALLOWABLE VALUES ASSIGNED BY THE BUILDING CODE OFFICIAL. COMPLETE DESIGN CALCULATIONS SHOWING INTERNAL LAYOUT, MEMBER FORCES AND STRESS CONTROL POINTS SHALL BE AVAILABLE UPON REQUEST FOR EACH TRUSS DESIGN. THE DESIGN OF THE TRUSSES ARE TO BE UNDER THE SUPERVISION OF A REGISTERED ENGINEER AND SHALL BE SEALED BY SAID ENGINEER.

- DRAWINGS SHALL BE FURNISHED BY THE MANUFACTURER SHOWING ALL CRITICAL DIMENSIONS FOR DETERMINING FIT AND PLACEMENT IN THE BUILDING AS WELL AS THE LOADS THE TRUSSES ARE DESIGNED TO SUPPORT. THESE DRAWINGS SHALL BE APPROVED PRIOR TO FABRICATION.
- THE TRUSSES IF STORED PRIOR TO ERECTION SHALL BE STORED IN A VERTICAL POSITION AND PROTECTED FROM THE WEATHER. THEY SHALL BE HANDLED WITH CARE SO THEY ARE NOT DAMAGED.
- THE TRUSSES ARE TO BE ERECTED AND INSTALLED IN ACCORDANCE WITH THE PLANS, THE APPROVED TRUSS DRAWINGS AND INSTALLATION SUGGESTIONS. TEMPORARY CONSTRUCTION LOADS WHICH CAUSE MEMBER STRESSES BEYOND DESIGN LIMITS ARE NOT PERMITTED. ERECTION BRACING IN ADDITION TO SPECIFIED BRIDGING IS TO BE PROVIDED TO KEEP THE TRUSS STRAIGHT AND PLUMB AS REQUIRED TO ASSURE ADEQUATE LATERAL SUPPORT FOR THE INDIVIDUAL TRUSS AND THE ENTIRE SYSTEM UNTIL THE SHEATHING MATERIAL HAS BEEN APPLIED.
- THE CONTRACTOR SHALL GIVE NOTIFICATION PRIOR TO ENCLOSING THE TRUSSES TO PROVIDE OPPORTUNITY FOR INSPECTION OF INSTALLATION.

WOOD FRAMING:

- ALL WALL SHEATHING SHALL BE 7/16" OSB, NAIL WITH 10d NAILS @ 6" O.C. AT ALL EDGE SUPPORTS AND 10d NAILS AT 12" O.C. AT ALL INTERMEDIATE SUPPORTS. PROVIDE STANDARD EDGE CLIPS AT MID-SPAN BETWEEN EACH SUPPORT. ALL ROOF DECK SHALL BE 5/8" OSB AND NAILED FOR THE ABOVE REQUIREMENTS.
- ALL ROOF JOISTS AND BEAMS TO BE GRADE STAMPED PER W.C.L.B. RULES AND BE OF SOUTHERN YELLOW PINE NO. 2 (K.D.) USED AT 15% MAX. MOISTURE CONTENT. FRAMING CONNECTORS SHALL BE SIMPSON STRONG-TIE AND SHALL BE BUILDING CODE APPROVED FOR THE TYPE OF INSTALLATION INDICATED.
- ALL 2 X STUD FRAMING SHALL BE S-P-F STUD GRADE USED AT 19% MAXIMUM MOISTURE CONTENT AND SHALL MEET THE MINIMUM REQUIREMENTS AS FOLLOWS:

Fb = 1500 PSI	#3 @ 12" O.C. E.W. TYPICAL
E = 1.6 X 10 ⁶ PSI	
- ALL PLATES, LEDGERS AND HEADERS SHALL BE S-P-F NO. 2, SURFACED DRY, USED AT 19% MAXIMUM MOISTURE CONTENT. FRAMING MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE SHALL BE WULMANIZED.
- ALL SILL PLATES SHALL BE SOUTHERN YELLOW PINE NO. 2 (K.D.) AND SHALL BE PRESSURE TREATED FOR MOISTURE RESISTANCE.
- ALL NAILING SHALL BE IN ACCORDANCE WITH THE 2006 INTERNATIONAL RESIDENTIAL CODE.
- ALL GLU-LAM BEAMS SHALL BE CONNECTED WITH STANDARD "STRONG-TIE" GLU-LAM CONNECTIONS, UNLESS DETAILED OTHERWISE.
- PROVIDE DOUBLE STUDS EACH SIDE OF ALL OPENINGS. PROVIDE DOUBLE JOIST HEADER ABOVE EVERY OPENING IN ACCORDANCE WITH THE FOLLOWING SCHEDULE.

OPENING WIDTH	HEADER SIZE
UP TO 2'-8"	2-2 X 8 OR 3-2 X 6
2'-8" TO 3'-8"	2-2 X 10 OR 3-2 X 8
3'-8" TO 6'-0"	2-2 X 12 OR 3-2 X 10

CODE & DESIGN SPECIFICATIONS:

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 - STRUCTURAL STEEL: 1989 SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS OF AISC (NINTH EDITION).
 - STRUCTURAL CONCRETE: 2002 BUILDING CODE FOR REINFORCED CONCRETE OF THE AMERICAN CONCRETE INSTITUTE (ACI 318-2002).
 - STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS, K-SERIES (SJI) AND AISC 1997.
- DESIGN LOADS: (LIVE LOAD) 2000 PSF

GENERAL CONDITIONS:

- THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND DETAILS BEFORE FABRICATION OR CONSTRUCTION AND REPORT ANY DISCREPANCIES OR OMISSIONS IN THE CONTRACT DOCUMENTS TO THE STRUCTURAL ENGINEER.
- THE CONTRACTOR SHALL COORDINATE ALL LEAVE-OUTS, SLEEVES AND OTHER SLAB PENETRATIONS BEFORE CONSTRUCTION.
- THE CONTRACTOR SHALL NOT PROCEED WITH FABRICATION OF STRUCTURAL ELEMENTS WITHOUT PRIOR REVIEW OF ALL SHOP DRAWINGS BY THE STRUCTURAL ENGINEER.
- THE STRUCTURAL ENGINEER'S REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS SHALL NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY FOR DEVIATIONS FROM THE CONTRACT DOCUMENTS, NOR FROM ANY ERRORS IN THE SHOP DRAWINGS.

FOUNDATION:

- THE FOUNDATION HAS BEEN DESIGNED USING THE INFORMATION AND RECOMMENDATIONS IN THE GEOTECHNICAL REPORT: PREPARED BY: ROCK ENGINEERING & TESTING LABORATORY, INC. REPORT NO.: 0209140 DATED: AUGUST 27, 2009
- BUILDING SITE SHALL BE DEFINED AS AN AREA TWO FEET BEYOND THE BUILDING LINE OR TWO FEET BEYOND THE SIDEWALK LIMITS, WHICHEVER IS FURTHER. BUILDING SITE SHALL BE EXCAVATED TO A DEPTH SUFFICIENT TO REMOVE ALL BRUSH AND VEGETATION. EXCAVATE 2'-0" MINIMUM. THE TOP SIX INCHES OF SUBGRADE SHALL BE SCARIFIED AND RECOMPACTED WHERE CUTTING IS REQUIRED.
- ANY FILL REQUIRED SHALL BE COMPAKTED IN EIGHT INCH LIFTS AT, OR ABOVE, ITS OPTIMUM MOISTURE CONTENT. ALL FILL REQUIRED TO RAISE TO PROPER SUBGRADE SHALL BE SELECT FILL.
- ALL SOFT SPOTS SHOULD BE OVER-EXCAVATED AND REPLACED WITH SELECT FILL COMPAKTED TO 95% RELATIVE COMPACTION.
- THE SELECT FILL SHOULD HAVE A P.I. BETWEEN 6 AND 15 AND A MAXIMUM LIQUID LIMIT OF 35. THE FILL SHOULD BE INSTALLED IN EIGHT INCH (MAXIMUM) LOOSE LIFTS AND UNIFORMLY COMPAKTED A MINIMUM OF 95% STANDARD PROCTOR DENSITY AT, OR SLIGHTLY ABOVE, ITS OPTIMUM MOISTURE CONTENT.
- FOOTING SHALL BEAR ON PROPERLY COMPAKTED FILL OR UNDISTURBED NATURAL GROUND WHERE SUITABLE. REMOVE ALL EXISTING FILL MATERIAL AT ISOLATED FOOTING. A QUALIFIED GEOTECHNICAL ENGINEER SHOULD BE PRESENT AT THE SITE TO DETERMINE WHICH AREAS WILL REQUIRE OVER-EXCAVATION AND RECOMPACT.

- DO NOT PAINT STEEL THAT IS EMBEDDED IN CONCRETE.

STRUCTURAL CONCRETE

1. TYPICAL CONCRETE SHALL BE HARDCOKE CONCRETE WITH A MINIMUM COMPRESSIVE STRENGTH AS SPECIFIED BELOW AT 28 DAYS. FIVE SACKS (470 LBS) MINIMUM OF CEMENT PER CUBIC YARD SHALL BE USED. NO CALCIUM CHLORIDE OR FLY ASH SHALL BE PERMITTED IN THE CONCRETE MIX. READY-MIXED CONCRETE SHALL CONFORM TO ASTM C94. MIX DESIGN SHALL INCLUDE LOCATION WHERE CONCRETE IS TO BE USED.
- SLAB-ON-GRADE 3000 PSI

2. CONCRETE SLUMPS SHALL BE FIVE INCHES MAXIMUM AND THREE INCHES MINIMUM.

3. PORTLAND CEMENT CEMENT SHALL CONFORM TO ASTM C150. AGGREGATE SHALL CONFORM TO ASTM C33. MAXIMUM AGGREGATE SIZE SHALL BE ONE INCH.

4. ALL CONCRETE WORK SHALL BE DONE IN STRICT ACCORDANCE WITH THE LATEST A.C.I. SPECIFICATIONS.

5. ALL REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 EXCEPT STIRRUPS AND TIES WHICH MAY BE GRADE 40.

6. REINFORCING STEEL SHALL BE DESIGNED, DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE LATEST A.C.I. "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" (A.C.I. 315) AND THE C.R.S.I. "RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS", LATEST EDITION.

7. REINFORCING SPlices SHALL LAP A MINIMUM OF 36 BAR DIAMETERS AND 18 INCHES MINIMUM UNLESS OTHERWISE NOTED. BAR SPlices AT LOCATIONS OF HIGH STRESSES ARE NOT PERMITTED. SPlices SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.

8. PROVIDE SPACERS, CHAIRS, BOLSTERS, TIES AND OTHER ACCESSORIES CONFORMING TO THE REQUIREMENTS OF THE C.R.S.I.

9. ALL GRADE BEAMS SHALL BE FORMED ON EXTERIOR SIDE.

STRUCTURAL STEEL:

1. STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", 1989 EDITION.
- STRUCTURAL STEEL TO CONFORM TO ASTM A992-50 GRADE STEEL UNLESS OTHERWISE NOTED.
- STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B.
- WELDING SHALL BE PER THE LATEST AWS STANDARDS WITH E70XX ELECTRODES. ALL WELDS IN FIELD TO BE LABORATORY INSPECTED. ALL FIELD WELDS TO BE DONE BY CERTIFIED WELDERS.
- PROVIDE FILLET WELDS AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.
- ALL WELDS TO EXISTING MEMBERS SHALL BE 3/16" FILLET WELDS ALL AROUND UNLESS NOTED OTHERWISE.
- BOLTS AND BOLTED CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATIONS FOR STRUCTURAL JOINTS USING A.S.T.M. A325 BOLTS OR A490 BOLTS" AS APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED JOINTS USING A325 BOLTS. USE FRiction TYPE BOLTS WITH WASHERS.
- SHEAR CAPACITY OF CONNECTIONS (BOTH WELDED AND BOLTED) SHALL NOT BE LESS THAN THE NOTED SHEAR OR 70% OF THE ALLOWABLE BEAM LOAD AS TABULATED IN THE A.I.S.C. MANUAL OF STEEL CONSTRUCTION, PART 2, FOR SIZE AND SPAN, WHICHEVER IS LARGER.
- UNLESS OTHERWISE NOTED, ALL STEEL SHALL BE SHOP PRIMED WITH FAST-CURING, LEAD AND CHROMATE-FREE, UNIVERSAL MODIFIED ALKYD PRIMER COMPLYING WITH PERFORMANCE REQUIREMENTS OF F.S. TT-P-664 SELECTED FOR GOOD RESISTANCE TO NORMAL ATMOSPHERIC CORROSION. ALL STEEL AND BRICK LINTEL ANGLES EXPOSED TO THE ELEMENTS SHALL BE GALVANIZED TO CONFORM WITH G-60.
- DO NOT PAINT STEEL THAT IS EMBEDDED IN CONCRETE.

REVISIONS:

DATE BY

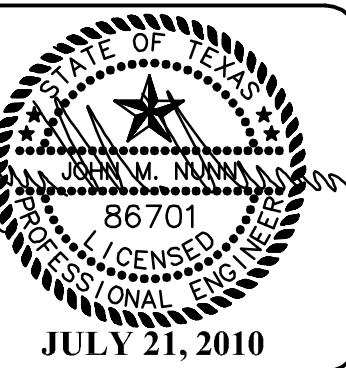
REVISIONS:

DATE

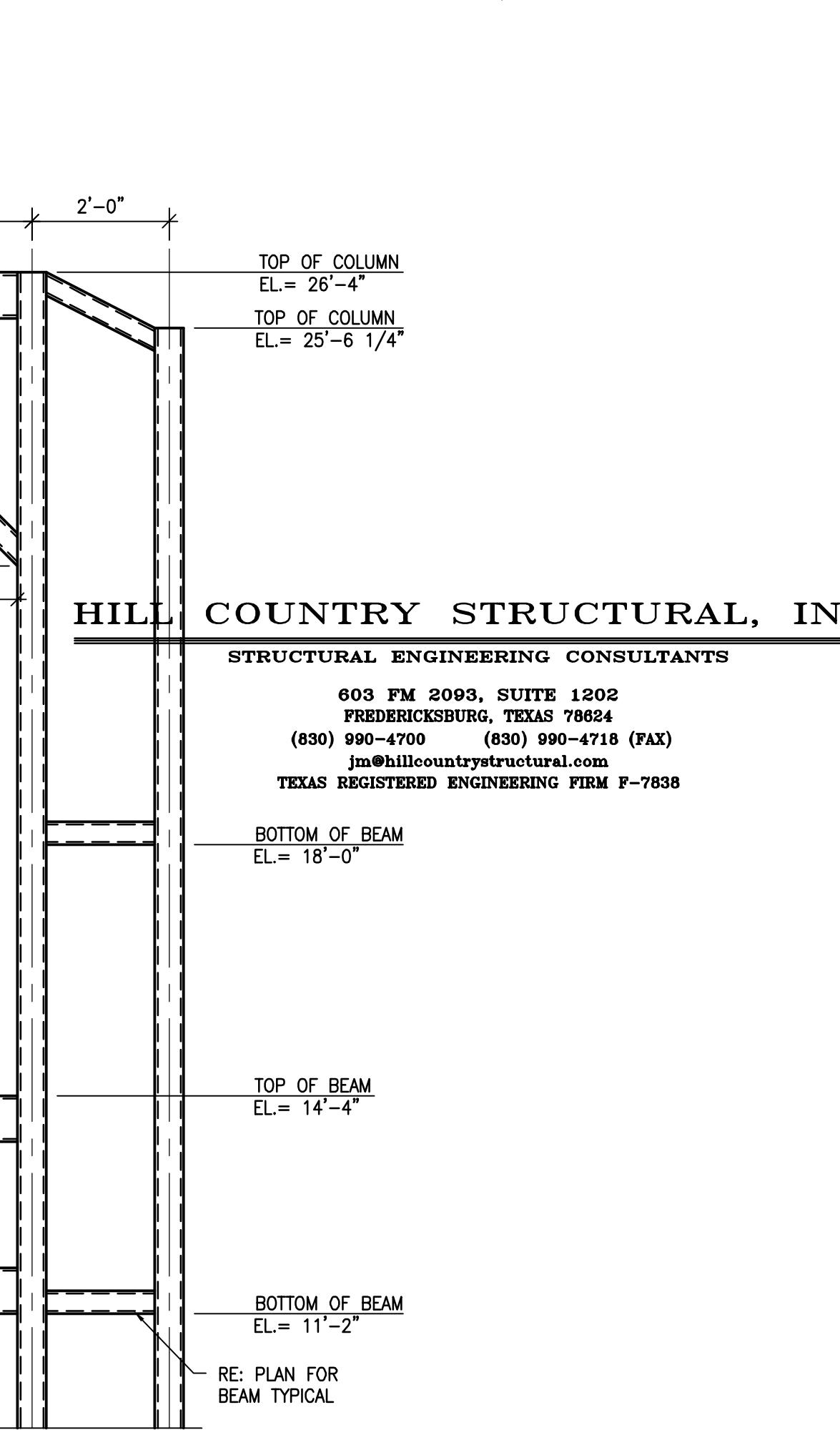
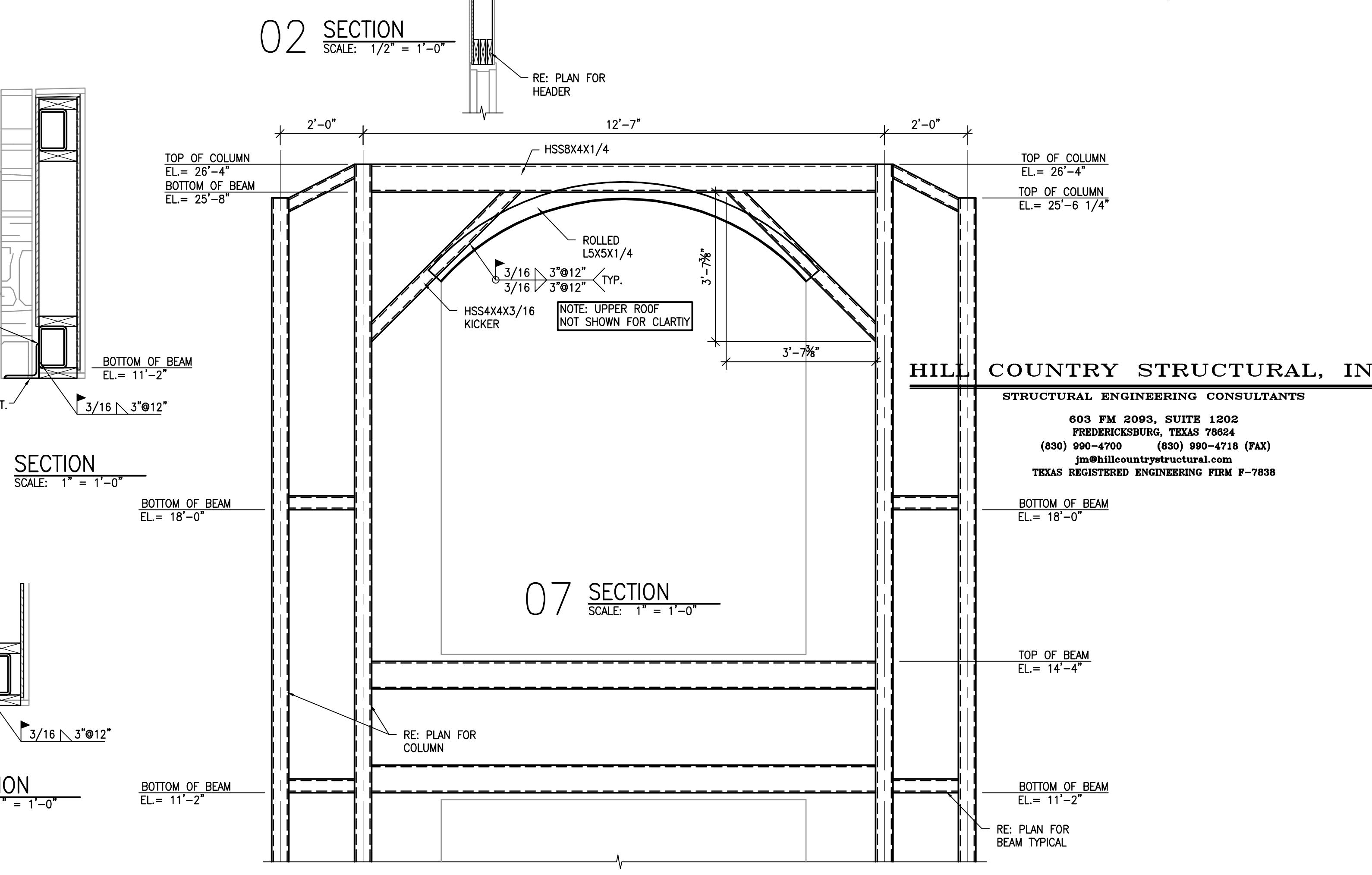
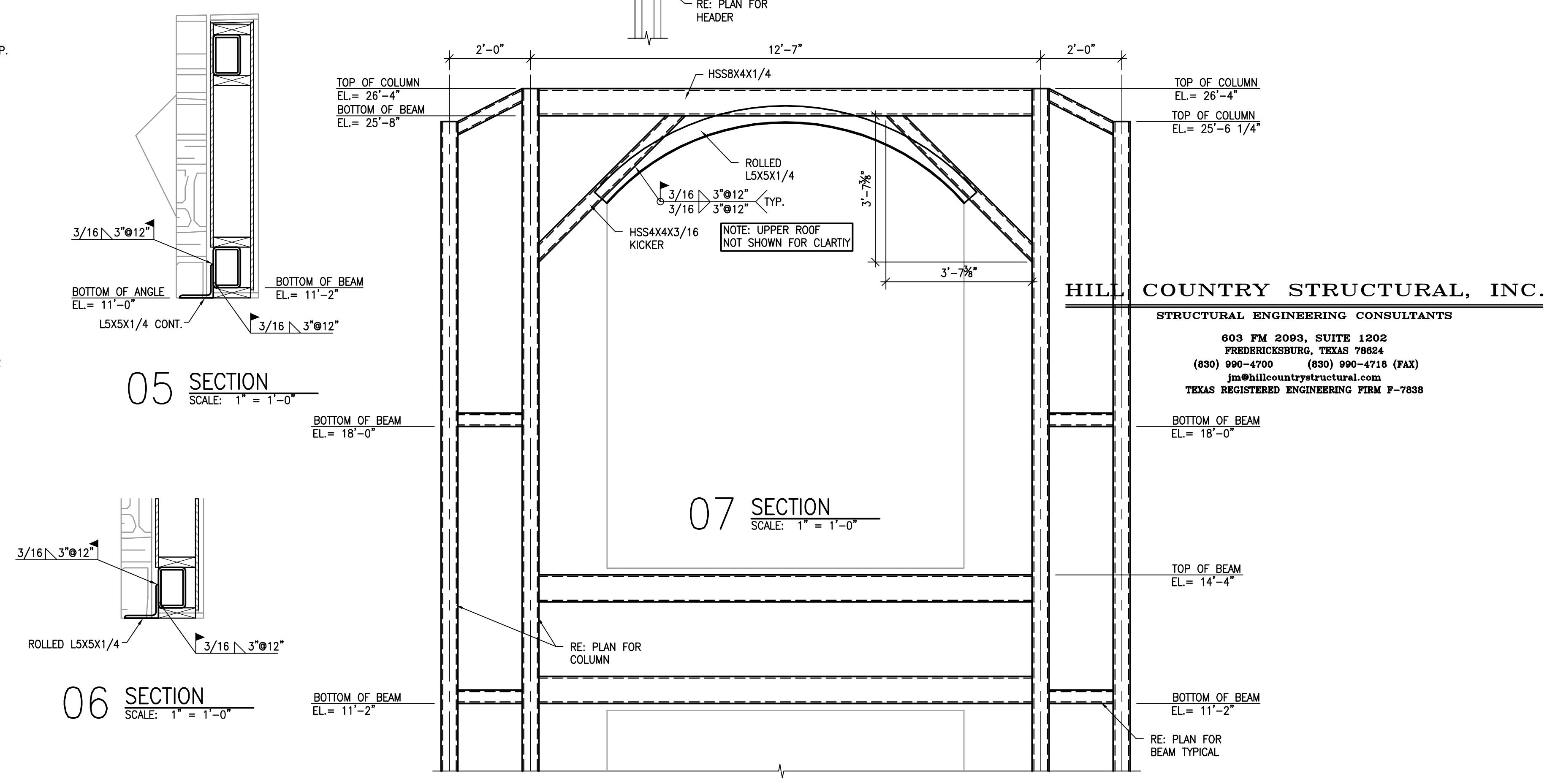
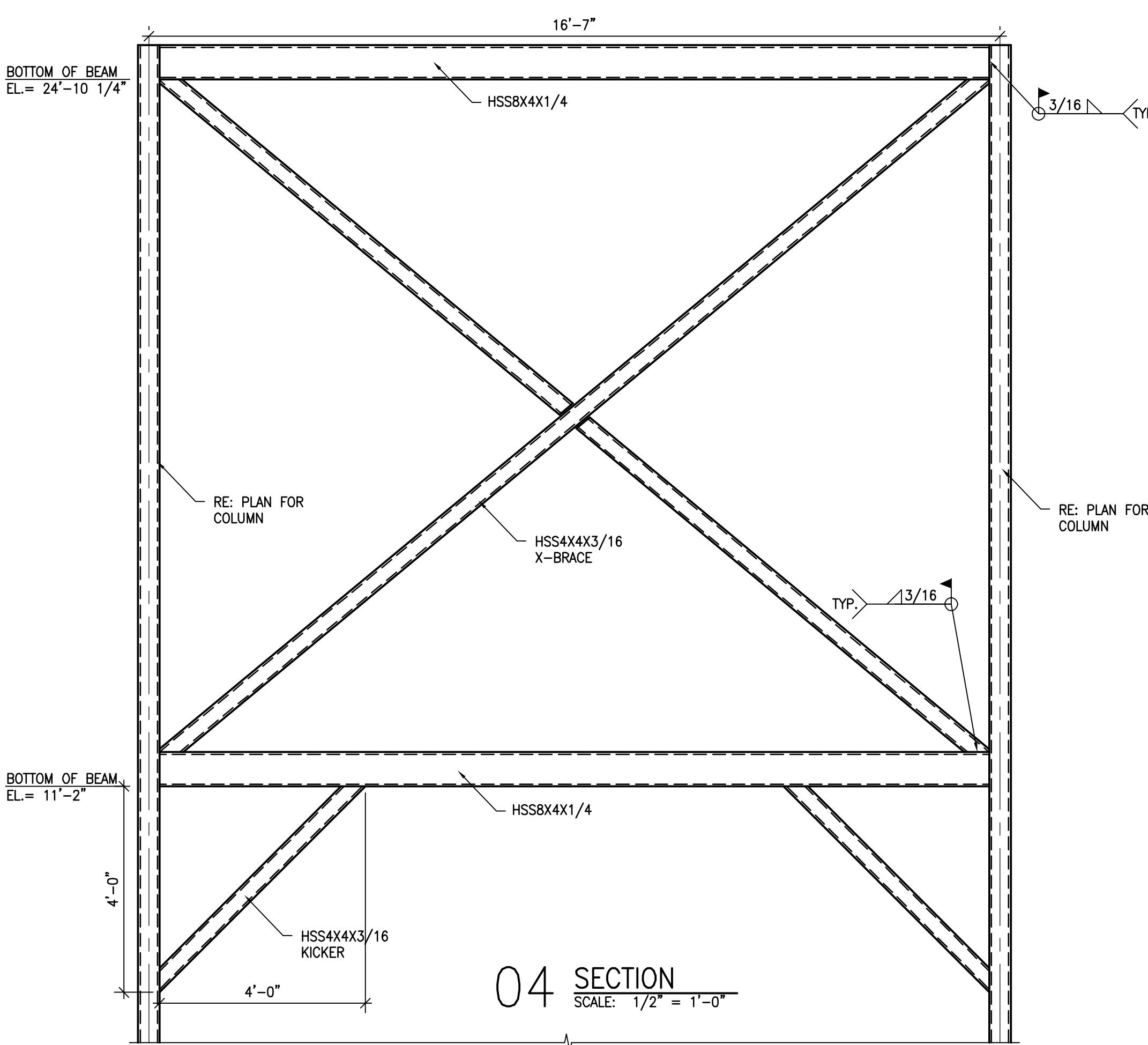
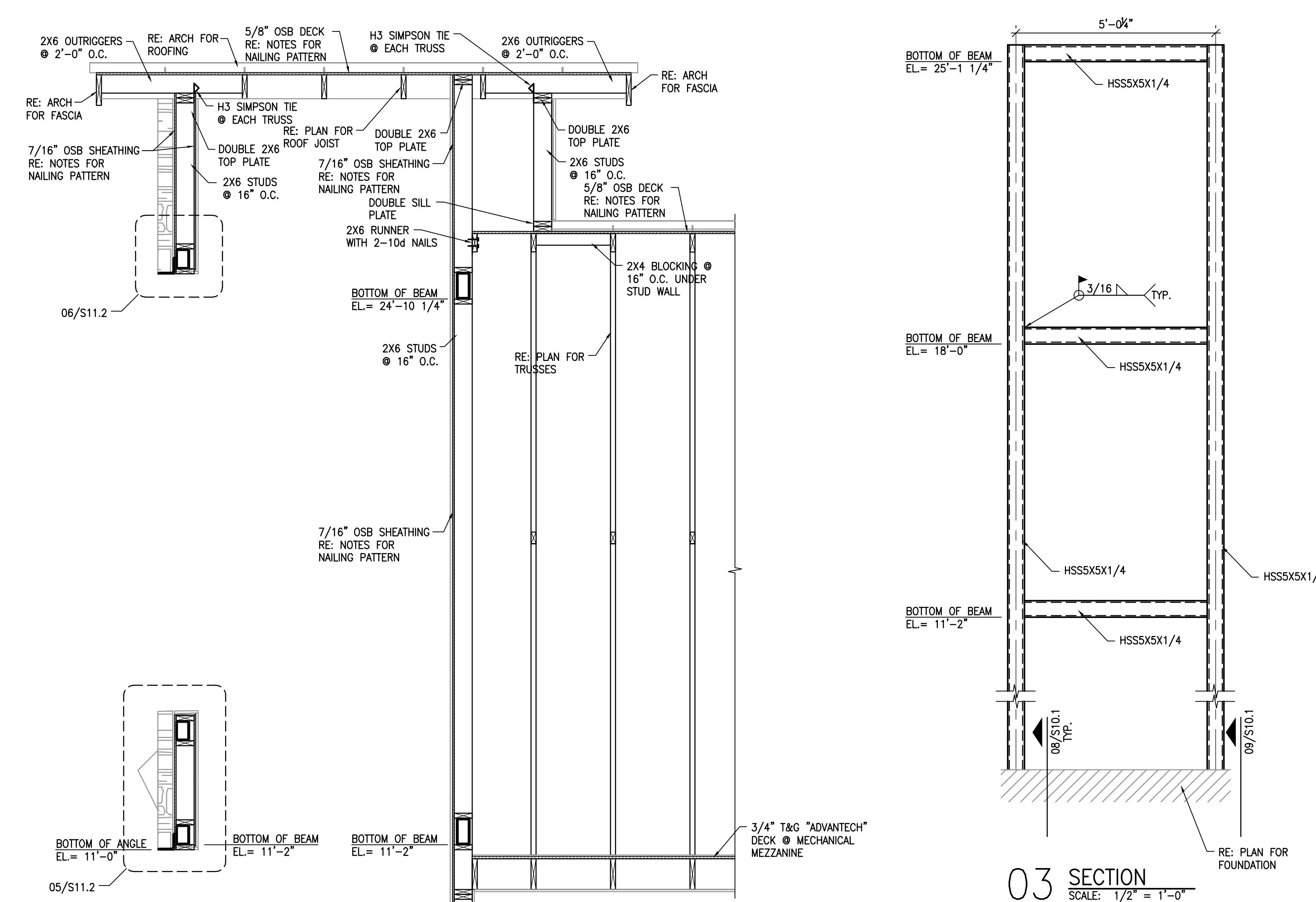
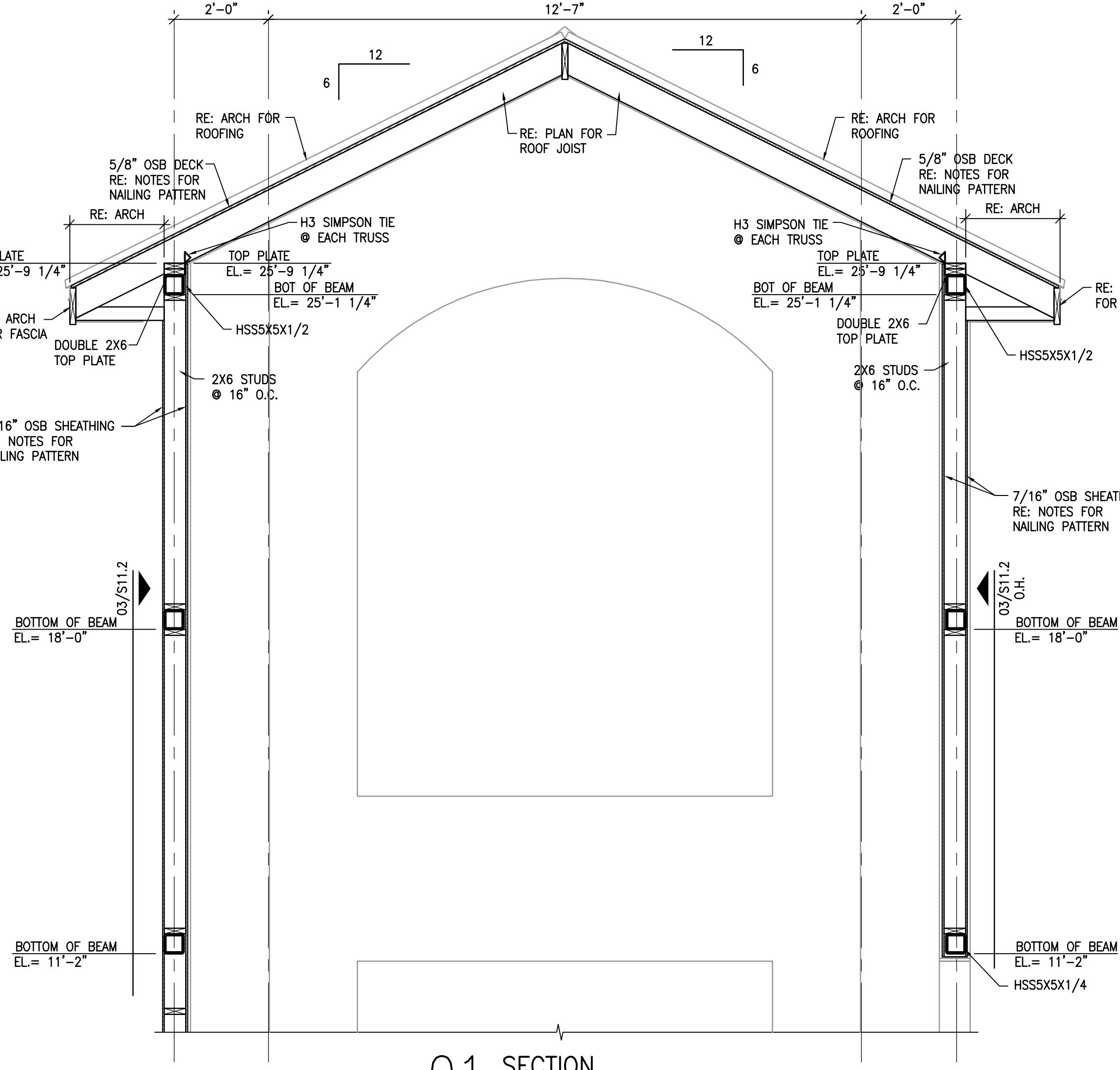
BY

TRUE VISION CHURCH
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CHAPEL FRAMING SECTIONS

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JOHN W. NUNN
86701
LICENCED PROFESSIONAL ENGINEER
JULY 21, 2010
SHEET NUMBER
S11.2



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jm@hillcountrystructural.com
TEXAS REGISTERED ENGINEERING FIRM F-7638

BOTTOM OF BEAM
EL = 18'-0"TOP OF BEAM
EL = 26'-4"TOP OF COLUMN
EL = 25'-6 1/4"TOP OF BEAM
EL = 14'-4"TOP OF BEAM
EL = 11'-2"

RE: PLAN FOR BEAM TYPICAL